# William Hilton Parkway Gateway Corridor Independent Review Advisory Committee Meeting 

March $27^{\text {th }}, 2024$


## Agenda

- Update on Project Progress
] Presentation of 2023 Existing Conditions (VISSIM Based Findings)
[ Presentation of 2045 No Build Conditions (VISSIM Based Findings)
- Presentation of Four Recommended Alternatives


## Update on Project Progress

## (Key Tasks Completed \& Continuing to Work on Since Last Meeting)

$\checkmark$ Completed 2045 No-Build analyses in Synchro.
$\checkmark$ Completed 2023 Existing \& 2045 No-Build VISSIM speed maps for the entire study area; videos focusing on congested locations during peak hours; calibration results in terms of travel time and volumes.
$\checkmark$ Completed preliminary traffic analyses \& Synchro modeling for proposed Alternatives \#1, \#2, \& \#3.
$\checkmark$ Continuing to finalize preliminary traffic analyses \& Synchro modeling for proposed Alternative \#4.
$\checkmark$ Evaluating Roadway, ROW, and Environmental impacts \& developing high-level, order of magnitude costs for Alternatives \#1, \#2, \#3, \& \#4.
$\checkmark$ Met with Town Staff on March 7, March 14, and March 20 to discuss project progress leading up to this March 27 Advisory Committee meeting.

## Update on Overall Project Progress (as of 3/22/24)

## Overall ~45\% Complete

$\square$

- Task \#1: Project Initiation and Coordination
- ~70\% Complete
- Task \#1 Subtasks On-Going:
> Remaining scheduled meetings with Town (Bi-Weekly) staff and Committee (Monthly)
> Continual review/reference previously completed existing reports and historical documentation
> Reviewing background information received to date related to Task 1.3.3 and Task 1.8; forthcoming brief technical memorandums and/or high level opinions of probable costs.


## Update on Overall Project Progress (as of 3/22/24)

Task \#2: Review of Travel Demand Model \& Evaluation of 2023 Existing Operating Conditions (Entire Study Area)

- $100 \%$ Complete

100\%

## Update on Overall Project Progress (as of 3/22/24)

- Task \#3: Traffic Model and Operational Update Modified Original Project Study Area
- ~35\% Complete
- Key Subtasks On-Going:
> Continuing to finalize preliminary traffic analyses \& Synchro modeling for proposed Alternative \#4.
> Evaluating Roadway, ROW, and Environmental impacts \& developing highlevel, order of magnitude costs for Alternatives \#1, \#2, \#3, \& \#4.
- Key Subtasks Forthcoming:
> Prepare Draft \& Final Summary of Findings Memos RE: Alternatives Analysis
> Participation in Public Open-House / Design Charrette (if required), to be led by Town staff


## 2023 Existing Results (VISSIM)

## - Travel Time results

| Corridor |  | Observed travel time (secs) |  | Simulated travel time (secs) |  | \% Difference |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM | AM | PM |
| Hilton Parkway between Moss Creek and Indigo Run | EB | 1108 | 672 | 1157 | 619 | 4.42\% | -7.91\% |
|  | WB | 605 | 1202 | 572 | 1252 | -5.50\% | 4.17\% |
| Hilton Parkway <br> @Moss Creek to <br> Cross Island <br> Parkway @Sea <br> Pine | SB | 1378 | 821 | 1356 | 857 | -1.58\% | 4.37\% |
|  | NB | 800 | 1516 | 826 | 1520 | 3.20\% | 0.25\% |

All travel time targets are satisfied and met

## 2023 Existing Results (VISSIM)

## - Volume results

| Location | Observed volume results <br> (vehicles) | Simulated volume results <br> (vehicles) | PM Difference |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |

All volume targets are satisfied and met

AM Speed Map

## 2023 Existing Results (VISSIM)

PM Speed Map

## 2023 Existing Results (VISSIM) - AM



## 2023 Existing Results (VISSIM) - PM



## 2045 No Build Results (VISSIM)

- Travel time results:.
- Travel time is expected to increase by up to approximately $62 \%$ when comparing Existing Conditions to 2045 No Build Conditions

| Corridor |  | Existing Simulated travel time (secs) |  | 2045 No Build Simulated travel time (secs) |  | \% Difference |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM | AM | PM |
| Hilton Parkway between Moss | EB | 1157 | 619 | 1579 | 642 | 36.47\% | 3.72\% |
| Creek and Indigo Run | WB | 572 | 1252 | 584 | 1544 | 2.10\% | 23.32\% |
| Hilton Parkway <br> @Moss Creek to Cross Island | SB | 1356 | 857 | 1984 | 873 | 46.31\% | 1.87\% |
| Parkway @Sea Pine | NB | 826 | 1520 | 823 | 2465 | -0.36\% | 62.17\% |

## 2045 No Build Results (VISSIM)

- Volume results:
- Note that the \% Difference with negative values are due to over-saturation. This results in vehicles not being able to enter the network to go through intersection.

| Location | Existing Simulated volume results (vehicles) |  | 2045 No Build Simulated volume results (vehicles) |  | \% Difference |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AM | PM | AM | PM | AM | PM |
| Hilton Parkway west of Squire Pope EB | 2970 | 2240 | 2783 | 2524 | -6.30\% | 12.68\% |
| Hilton Parkway west of Squire Pope WB | 1650 | 2898 | 1843 | 2754 | 11.70\% | -4.97\% |
| Hilton Parkway east of Spanish Wells EB | 2929 | 1955 | 2748 | 2198 | -6.18\% | 12.43\% |
| Hilton Parkway east of Spanish Wells WB | 1423 | 2614 | 1570 | 2413 | 10.33\% | -7.69\% |
| Cross Island Parkway north of Marshland Rd NB | 964 | 1371 | 1065 | 919 | 10.48\% | -32.97\% |
| Cross Island Parkway north of Marshland Rd SB | 1486 | 1167 | 1440 | 1299 | -3.10\% | 11.31\% |

2045 No Build Results
(VISSIM)

AM Speed Map

2045 No Build Results (VISSIM)

PM Speed Map

## 2045 No Build Results (VISSIM) - AM



## 2045 No Build Results (VISSIM) - PM



## Existing and 2045 No Build Conclusions

- As shown, significant congestion currently exists throughout the study area. This congestion is expected to be exacerbated by 2045 with the forecasted growth.
- 2023 existing traffic volumes would need to be reduced by approximately $30 \%$ for the current 4 -lane configuration to operate acceptably.
- Based on an annual growth rate of $0.56 \%, 2045$ traffic volumes would need to be reduced by approximately $40 \%$ for the current 4-lane configuration to operate acceptably.
- Therefore, it is our recommendation that a six-lane section is necessary to accommodate the projected traffic volumes.


## Second Bridge

- A second bridge has been discussed locally as a potential solution
- A new bridge could take up to 20 years to construct once all the studies, planning, design, and construction is complete
- The operating conditions today already show significant congestion
- Our focus is to mitigate the existing and forecasted conditions, and as shown in 20 years, the congestion along the corridor will be significant
- It is our opinion that a 6-lane section is needed independently of a second bridge


## Town of Hilton Head Island Gateway Corridor Project Guiding Principles

- Lochmueller Group studied four alternatives to help improve the US 278 corridor. In determining the most beneficial and appropriate alternatives, the following guiding principles were top of mind:
- Fix the transportation issues in the corridor in a way that improves the safety and quality of life for all residents, workers, and visitors to Hilton Head Island
- Address transportation needs for natural disasters and resiliency of island access
- Consider future transportation alternatives
- Improve the safety and quality of life for the residents of the neighborhoods and businesses directly impacted by the US 278 corridor
- Stoney Community
- Neighborhoods on Jenkins and Hog Islands (including but not limited to Windmill Harbor)
- Have gateway to and from Hilton Head Island that the region will be proud of:
- Aesthetically pleasing and reflecting the Hilton Head Island/Low County values
- Safe and functional pathways for pedestrians and cyclists
- Minimizes environmental impacts and enhances the national asset of Pinckney Island


## Recommended Alternative Improvements

- Alternative 1: SCDOT Modified Recommended Preferred
- Alternative 2: Bowties at Squire Pope and Spanish Wells
- Alternative 3: Echelon / Center Turn Overpass
- Grade Separated Intersection at either Squire Pope or Spanish Wells
- Alternative 4: Elevated Bypass


## Alternative 1 - SCDOT Modified Recommended Preferred



## Alternative 1 - SCDOT Modified Recommended Preferred

- This alternative considers the Modified Recommended Preferred Alternative from SCDOT's concept in addition to the following updates:
- Adaptive signals in place
- $0.56 \%$ growth rate
- Improvements to Gumtree
- At-grade intersection
- Complete reconfiguration is expected to improve the efficiency of the intersection
- Dual left-turn lanes at the westbound, northbound, and southbound approaches were included


## Alternative 1 - SCDOT Modified Recommended Preferred

- Summary of forecasted operating conditions:
- Overall, the study intersections are expected to operate acceptably under Alternative 1
- Each signalized intersection is expected to operate with a LOS D or better overall
- Side-street and unsignalized approaches may still experience long delays
- Travel times were performed in Synchro along the Hilton Parkway corridor between Moss Creek and Indigo Run.
- When compared to the 2045 No Build Scenario, the following changes in travel time were calculated:

| Scenario |  | Difference in Travel Time (seconds) when Compared to 2045 No Build |  | \% Difference |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |
| 2045 Alternative 1: SCDOT Modified Recommended | EB | -109 | +18 | -16.1\% | 2.7\% |
| Preferred | WB | +43 | -276 | 6.3\% | -34.2\% |

## Alternative 1 - SCDOT Modified Recommended Preferred

In order to potentially reduce the required ROW, Lochmueller evaluated if any modifications to Alternative 1 could be implemented. The following modification were considered:

- Reduce the dual eastbound left-turn lanes at Squire Pope to one left-turn lane
- Results:
- The movement would be over capacity
- The queue length would be expected to increase from approximately 7 vehicles to 23 vehicles ( +16 vehicles) during the PM peak hour
- The delays at the movement are expected to increase from approximately 75 seconds to 214 seconds ( +139 seconds.) during the PM peak hour
- Reduce the dual southbound right-turn lanes at Squire Pope to one right-turn lane
- Results:
- The movement would be over capacity
- The queue length would be expected to increase from approximately 9 vehicles to 24 vehicles ( +15 vehicles) during the PM peak hour.
- The delays at the movement are expected to increase from approximately 50 seconds to 116 seconds ( +66 seconds.) during the PM peak hour


Alternative 1


Potential Changes to Decrease Needed ROW

Alternative 2 - Bowties at Squire Pope and Spanish Wells


## Alternative 2 - Bowties at Squire Pope and Spanish Wells

- This alternative removes left-turns from Squire Pope and Spanish Wells
- Vehicles must navigate through bowtie intersections to make their left-turn movements


Hilton Pkwy at Squire Pope


Hilton Pkwy at Spanish Wells

## Alternative 2 - Bowties at Squire Pope and Spanish Wells

- Pros of Bowties
- Improves safety as fewer conflicting vehicle and pedestrian movements are allowed
- Bowtie intersections have 12 fewer conflict points when compared to traditional intersections
- Encourages lower vehicle speeds through the roundabouts
- Reduces the size of intersections as left-turn lanes are removed
- Increases the efficiency of the signalized intersections as more of the signal time can go to the main through movements, improving traffic operations
- Cons of Bowties
- Requires a change in decision making for drivers as all left-turn movements would become rightturns at Squire Pope and Spanish wells
- Increases the amount of travel time for left-turning vehicles
- Requires additional ROW for the roundabouts


## Alternative 2 - Bowties at Squire Pope and Spanish Wells

- Summary of forecasted operating conditions:
- The same improvements to study intersections other than Squire Pope and Spanish Wells (including Gumtree) that were included in Alternative 1 were also included in Alternative 2
- Bowties at Squire Pope and Spanish Wells is expected to improve operations throughout the study area
- Overall, the study intersections are expected to operate acceptably under Alternative 2
- Each signalized intersection is expected to operate with a LOS D or better overall
- $\quad$ Side-street and unsignalized approaches may still experience long delays
- Travel times were performed in Synchro along the Hilton Parkway corridor between Moss Creek and Indigo Run.
- When compared to the 2045 No Build Scenario, the following changes in travel time were calculated:

| Scenario |  | Difference in Travel Time (seconds) when Compared to 2045 No Build |  | \% Difference |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |
| 2045 Alternative 2: Bowties at Squire Pope and | EB | -104 | -39 | -15.3\% | -6.6\% |
| Spanish Wells | WB | +9 | -290 | 1.4\% | -36.4\% |

## Alternative 2 - Bowties at Squire Pope and Spanish Wells

- The expected changes in travel time for the displaced left-turn movements are as follows:

| Squire Pope |  |  |  |  | Spanish Wells |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| AM Peak Hour | PM Peak Hour |  | AM Peak Hour | PM Peak Hour |  |  |
| Eastbound Left-Turn | 63 seconds | 42 seconds | Eastbound Left-Turn | -10 seconds | -26 seconds |  |
| Westbound Left-Turn | 82 seconds | 81 seconds | Westbound Left-Turn | -13 seconds | -7 seconds |  |
| Northbound Left-Turn | 101 seconds | 120 seconds | Northbound Left-Turn | 89 seconds | -42 seconds |  |
| Southbound Left-Turn | 44 seconds | 4 seconds | Southbound Left-Turn | 43 seconds | 61 seconds |  |

## Alternative 3 - Echelon / Center Turn Overpass



Echelon


Center-Turn Overpass

## Alternative 3 - Echelon



## Alternative 3 - Echelon

- Pros of Echelon
- Improves safety as fewer conflicting vehicle and pedestrian movements are allowed
- Echelon intersections have 10 fewer conflict points when compared to traditional intersections
- Does not require a change in decision making for drivers as all movements would remain at Squire Pope and Spanish wells
- Increases the efficiency of the signalized intersections as more of the signal time can go to the main through movements, improving traffic operations
- While each approach would likely need the same lane configurations shown in Alternative 1, only two approaches would conflict, which would reduce the amount of ROW for each respective grade
- Cons of Echelon
- Requires grade separation of intersections
- Overall, large intersections would likely remain as all movements (leftturn, through, and right-turn) are still allowed


## Alternative 3 - Center Turn Overpass



## Alternative 3 - Center Turn Overpass

- Pros of Center Turn Overpass
- Improves safety as fewer conflicting vehicle and pedestrian movements are allowed
- Center Turn Overpass intersections have 8 fewer conflict points when compared to traditional intersections
- Increases the efficiency of the signalized intersections as more of the signal time can go to the main through movements, improving traffic operations
- Reduces the size of intersections as left-turn lanes are removed at-grade
- Cons of Center Turn Overpass
- Requires grade separation of intersections
- Overall, large intersections would likely remain as all movements (leftturn, through, and right-turn) are still allowed


## Alternative 3 - Echelon / Center Turn Overpass

- Summary of forecasted operating conditions:
- The same improvements to study intersections other than Squire Pope and Spanish Wells (including Gumtree) that were included in Alternative 1 were also included in Alternative 3
- Overall, the study intersections are expected to operate acceptably under Alternative 3
- Each signalized intersection is expected to operate with a LOS D or better overall
- Side-street and unsignalized approaches may still experience long delays
- The long queues and delay that are expected under the 2045 No Build Scenario would be expected to be improved overall


## Alternative 3 - Echelon / Center Turn Overpass

- Travel times were performed in Synchro along the Hilton Parkway corridor between Moss Creek and Indigo Run.
- When compared to the 2045 No Build Scenario, the following changes in travel time were calculated:

| Scenario |  | Difference in Travel Time (seconds) when Compared to 2045 No Build |  | \% Difference from 2045 No Build |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |
| 2045 Alternative 3: Echelon at Squire Pope | EB | -191 | -68 | -32.1\% | -12.1\% |
|  | WB | -79 | -407 | -13.9\% | -60.0\% |
| 2045 Alternative 3: Echelon at Spanish Wells | EB | -140 | -11 | -21.6\% | -1.8\% |
|  | WB | -15 | -331 | -2.3\% | -44.0\% |
| 2045 Alternative 3: Center Turn Overpass at Squire Pope | EB | -122 | +23 | -18.4\% | 3.5\% |
|  | WB | -21 | -319 | -3.3\% | -41.6\% |
| 2045 Alternative 3: Center Turn Overpass at Spanish Wells | EB | -134 | -17 | -20.6\% | -2.9\% |
|  | WB | -16 | -339 | -2.5\% | -45.4\% |

## Alternative 4 - Elevated Bypass



Note: The alignment shown is preliminary. Additional information such as operating results and tie-in points will be further defined in the April $10^{\text {th }}$ meeting

## Alternative 4 - Elevated Bypass



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## Alternative 4 - Elevated Bypass



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## Alternative 4 - Elevated Bypass

- Pros of Elevated Bypass
- Improves safety as fewer through vehicles would remain at grade, reducing the conflicts between vehicles and pedestrians
- Increases the efficiency of the atgrade signalized intersections
- $\quad$ Significantly reduces the size of atgrade intersections
- Provides room for growth along the corridor
- Cons of Elevated Bypass
- Most expensive alternative
- Results in the largest construction impact
- Requires grade separation
- Requires a change in decision making as drivers looking to go through the local intersections of Squire Pope, Old Wildhorse Drive, and Spanish Wells would need to enter the bypass.
- However, full access would still be provided at-grade


## Next Steps

- Each alternative will be compared to the same categories as shown in the draft matrix
- The scores shown are examples for illustrative purpose only. The final matrix will be provided in the April 10 meeting

|  | Traffic Operations (Throughput and Traffic Simulations) | ROW Impact | Environmental Impact | Safety | Bike/Pedestrian Impact | Community and Social Impact | Aesthetic | Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Alternative 1 | 1 | 2 |  |  |  |  |  |  |
| Alternative 2 | 2 | 2 |  |  |  |  |  |  |
| Alternative 3 | 3 |  |  |  |  |  |  |  |
| Alternative 4 | 4 |  |  |  |  |  |  |  |

1 = Highest Ranking

## QUESTIONS?



LOCHMUELLER https://wwww.islandpacket.com/news/local/traffic/article235209867.html

## APPENDIX


https://wwww.islandpacket.com/news/local/traffic/article235209867.html

## Travel Time Comparison

| Scenario |  | Difference in Travel Time (seconds) when Compared to 2045 No Build |  | \% Difference |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM | PM | AM | PM |
| 2045 Alternative 1: SCDOT Modified Recommended Preferred | EB | -109 | +18 | -16.1\% | 2.7\% |
|  | WB | +43 | -276 | 6.3\% | -34.2\% |
| 2045 Alternative 2: Bowties at Squire Pope and Spanish Wells | EB | -104 | -39 | -15.3\% | -6.6\% |
|  | WB | +9 | -290 | 1.4\% | -36.4\% |
| 2045 Alternative 3: Echelon at Squire Pope | EB | -191 | -68 | -32.1\% | -12.1\% |
|  | WB | -79 | -407 | -13.9\% | -60.0\% |
| 2045 Alternative 3: Echelon at Spanish Wells | EB | -140 | -11 | -21.6\% | -1.8\% |
|  | WB | -15 | -331 | -2.3\% | -44.0\% |
| 2045 Alternative 3: Center Turn Overpass at Squire Pope | EB | -122 | +23 | -18.4\% | 3.5\% |
|  | WB | -21 | -319 | -3.3\% | -41.6\% |
| 2045 Alternative 3: Center Turn Overpass at Spanish Wells | EB | -134 | -17 | -20.6\% | -2.9\% |
|  | WB | -16 | -339 | -2.5\% | -45.4\% |

Note that the travel times were completed in Synchro for the Hilton Parkway corridor between Moss Creek and Indigo Run

## 2023 Existing VISSIM Results

AM Peak Hour Volumes PM Peak Hour Volumes

## 2023 Existing Traffic Volumes




AM Peak Hour Volumes PM Peak Hour Volumes

## 2023 Existing Traffic Volumes



(6)


AM Peak Hour Volumes PM Peak Hour Volumes

## 2023 Existing Traffic Volumes





AM Peak Hour Volumes PM Peak Hour Volumes

## 2023 Existing Traffic Volumes



| (16) | $\begin{aligned} & \leftarrow 41 \\ & \leftarrow 1012 \end{aligned}$ |
| :---: | :---: |
| $\begin{array}{\|r\|r\|} \hline \text { US } 278 & \\ 1589 & \longrightarrow \\ 92 & \end{array}$ | $\prod_{0}^{0}$ |


| (17) | $\begin{aligned} & \leftarrow 304 \\ & \leftarrow 1366 \\ & \leftarrow 51 \end{aligned}$ |
| :---: | :---: |
| $$ |  |


| (16) $\stackrel{i}{j}$ | $\begin{aligned} & \leftarrow 10 \\ & \leftarrow 1531 \end{aligned}$ |
| :---: | :---: |
| $\begin{array}{\|r\|} \hline \text { US } 278 \\ 1304 \\ 5 \end{array}$ | ¢ |

AM Peak Hour Volumes PM Peak Hour Volumes

## 2023 Existing Traffic Volumes





AM Peak Hour Volumes PM Peak Hour Volumes

## 2023 Existing Traffic Volumes




AM Peak Hour Volumes PM Peak Hour Volumes

## 2023 Existing Traffic Volumes




AM Peak Hour Volumes PM Peak Hour Volumes

## 2023 Existing Traffic Volumes





## 2023 Existing Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{ Max Queue, ft $\}$ |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 1: Buckingham Plantation Dr \& Bluffton Pkwy (signal) |  |  |
| Overall Intersection | B (12.2) | B (11.4) |
| Eastbound Approach | B (12.5) [43] \{324\} | B (11.7) [<25] \{178\} |
| Westbound Approach | A (8.3) [<25] \{171\} | A (9.3) [<25] \{234\} |
| Northbound Approach | D (38.1) [<25] \{73\} | C (32.2) [<25] \{65\} |
| Southbound Approach | A (9.0) [<25] \{68\} | B (12.6) [<25] \{84\} |
| 2: Buckingham Plantation Dr/Moss Creek Dr \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | D (47.2) | B (19.5) |
| Eastbound Approach | E (70.7) [807] \{1620\} | B (15.6) [65] \{522\} |
| Westbound Approach | B (14.4) [35] \{353\} | B (19.1) [100] \{633\} |
| Northbound Approach | D (35.9) [37] \{263\} | D (53.6) [<25] \{104\} |
| Southbound Approach | D (42.4) [<25] \{92\} | C (33.6) [29] \{161\} |
| 3: Salt Marsh Dr/Moss Creek Village \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | E (35.3) [720] \{1302\} | C (19.7) [<25] \{47\} |
| Westbound Left Turn | F (414.9) [37] \{107\} | B (11.2) [<25] \{41\} |
| Northbound Approach | B (10.1) [<25] \{61\} | A (7.3) [<25] \{55\} |
| Southbound Approach | C (19.7) [<25] \{76\} | A (8.0) $[<25]\{73\}$ |
| 4: Fording Island Rd Ext \& Hilton Pkwy (un-signalized) |  |  |
| Westbound Left Turn | E (40.7) [46] \{102\} | C (16.8) [<25] \{28\} |
| Northbound Approach | F (591.0) [120] \{186\} | $\mathrm{C}(17.5)[<25]\{41\}$ |



Northound Approach F (591.0) [120] \{186\} C (17.5) [<25] \{41\}

## 2023 Existing Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft$\}$ |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 5: Boat Landing Driveway/Wildlife Refuge Driveway \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | C (21.4) [691] \{1658\} | D (33.1) [<25] \{<25\} |
| Westbound Left Turn | No volumes observed | C (20.9) $[<25]\{<25\}$ |
| Northbound Approach | F (270.5) [<25] \{38\} | C (20.8) [<25] \{46\} |
| Southbound Approach | No volumes observed | $\mathrm{C}(18.2)[<25]\{39\}$ |
| 6: Blue Heron Point Rd \& Hilton Pkwy (un-signalized) |  |  |
| Westbound Left Turn | No volumes observed | D (27.7) [<25] \{<25\} |
| Northbound Approach | F (1,270.4) [123] \{228\} | C (23.8) [<25] \{60\} |
| 7: Crosstree Dr (Windmill Harbour) \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | B (10.4) | A (8.2) |
| Eastbound Approach | A (8.4) [335] \{1093\} | A (5.4) [62] \{890\} |
| Westbound Approach | B (10.7) [53] \{655\} | A (7.5) [292] \{1626\} |
| Northbound Approach | E (67.8) [30] \{164\} | E (79.0) [52] \{214\} |
| Southbound Approach | B (17.3) [<25] \{<25\} | A (0.0) [<25] \{<25\} |
| 8: Jenkins Rd \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | A (8.2) $[<25]\{<25\}$ | F (98.0) [<25] \{54\} |
| Southbound Approach | F (51.8) [<25] \{44\} | F (109.4) [<25] \{92\} |



## 2023 Existing Results (VISSIM)



## 2023 Existing Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft \} |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 13: Jarvis Park Rd/Wilborn Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (21.8) | C (25.8) |
| Eastbound Approach | B (17.1) [178] \{929\} | B (13.8) [67] \{754\} |
| Westbound Approach | B (17.9) [73] \{543\} | C (34.2) [668] \{1285\} |
| Northbound Approach | F (81.1) [45] \{168\} | E (77.4) [43] \{186\} |
| Southbound Approach | D (45.4) [72] \{233\} | $\mathrm{C}(21.7)[28]\{120\}$ |
| 14: Pembroke Dr/Museum St \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | B (18.7) | C (33.7) |
| Eastbound Approach | B (12.6) [134] \{991\} | B (14.3) [76] \{754\} |
| Westbound Approach | B (12.7) [39] \{443\} | D (42.4) [645] \{1261\} |
| Northbound Approach | E (72.0) [84] \{274\} | E (69.4) [93] \{310\} |
| Southbound Approach | D (37.3) [<25] \{91\} | D (42.2) [<25] \{115\} |
| 15: Central Ave \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | A (8.5) $[<25]\{<25\}$ | $\mathrm{F}(55.9)[<25]\{50\}$ |
| Westbound Left Turn | C (16.5) [<25] $\{<25\}$ | A (9.7) [<25] \{<25\} |
| Northbound Right Turn | C (21.6) [<25] \{27\} | B (12.4) [<25] \{32\} |
| Southbound Right Turn | B (11.3) [<25] \{41\} | F (165.1) [<25] \{65\} |
| 16: Hatton Pl/Merchant St \& Hilton Pkwy (un-signalized) |  |  |
| Northbound Right Turn | B (11.5) [<25] \{25\} | A (7.7) [<25] \{61\} |
| Southbound Right Turn | B (10.6) [<25] \{46\} | F (93.1) [<25] \{72\} |
| 17: Indigo Run Dr/Whooping Crane Way \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (23.3) | D (44.1) |
| Eastbound Approach | В (18.9) [115] \{848\} | C (30.6) [116] \{772\} |
| Westbound Approach | B (15.3) [40] \{333\} | D (44.8) [592] \{1536\} |
| Northbound Approach | D (54.0) [29] \{103\} | E (58.6) [46] \{190\} |
| Southbound Approach | $\mathrm{D}(49.7)$ [52] \{187\} | E (73.4) [83] \{324\} |



## 2023 Existing Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft\} |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 18: Cross Island Pkwy SB Ramp/Gumtree Rd \& Honey Horn Rd (un-signalized) |  |  |
| Eastbound Approach | A (9.1) [<25] \{45\} | B (10.9) [<25] \{61\} |
| 19: Cross Island Pkwy SB Ramp \& Marshland Rd (un-signalized) |  |  |
| Westbound Left Turn | A (3.7) [<25] \{63\} | A (2.1) [ $<25]\{60\}$ |
| Southbound Left Turn | A (8.8) $[<25]\{47\}$ | B (10.6) [<25] \{77\} |
| Southbound Right Turn | A (6.1) [<25] \{80\} | A (7.9) [<25] $\{110\}$ |
| 20: Cross Island Pkwy NB Ramp \& Marshland Rd (un-signalized) |  |  |
| Eastbound Left Turn | A (1.3) [ 225$]\{36\}$ | A (1.4) $[<25]\{<25\}$ |
| Northbound Left Turn | B (10.8) [<25] \{69\} | C (21.3) [31] \{193\} |
| Northbound Right Turn | A (6.0) [<25] \{94\} | A (7.0) [46] \{218\} |



## 2023 Existing Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft $\}$ |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 21: Palmetto Bay Rd \& Bay Pines Rd (un-signalized) |  |  |
| Eastbound Left Turn | No volumes observed | E (40.7) [<25] \{33\} |
| Eastbound Right Turn | B (14.3) [<25] \{25\} | B (12.5) [<25] \{31\} |
| Northbound Left Turn | A (9.0) [<25] \{<25\} | A (7.1) [<25] \{34\} |
| 22: Palmetto Bay Rd \& Point Comfort Rd/Arrow Rd (signal) |  |  |
| Overall Intersection | B (13.2) | B (16.9) |
| Eastbound Approach | C (33.3) [52] \{191\} | C (32.0) [37] \{175\} |
| Westbound Approach | B (18.8) [<25] \{106\} | C (23.2) [56] \{261\} |
| Northbound Approach | B (10.5) [29] \{261\} | B (15.7) [92] \{612\} |
| Southbound Approach | B (10.9) [94] \{873\} | B (14.2) [67] \{606\} |
| 23: Palmetto Bay Rd \& Genesta St (un-signalized) |  |  |
| Westbound Approach | C (17.1) $[<25]\{50\}$ | C (18.1) $[<25]\{52\}$ |
| Southbound Left Turn | A (3.2) [<25] \{<25\} | B (13.2) [<25] \{<25\} |
| 24: Palmetto Bay Rd \& Palmetto Business Park Rd (un-signalized) |  |  |
| Westbound Approach | C (18.0) [<25] \{53\} | C (17.4) $[<25]\{53\}$ |
| Southbound Left Turn | A (4.0) [<25] \{37\} | A (7.5) [<25] $\{<25\}$ |
| 25: Palmetto Bay Rd \& Bow Cir (un-signalized) |  |  |
| Westbound Left Turn | $\mathrm{D}(27.5)[<25]\{<25\}$ | $\mathrm{D}(30.0)[<25]\{35\}$ |
| Westbound Right Turn | A (8.4) [<25] $\{58\}$ | B (13.1) [<25] \{68\} |
| Southbound Left Turn | A (3.6) [<25] \{40\} | B (11.0) [<25] \{26\} |
| 26: Palmetto Bay Rd \& Archer Rd (un-signalized) |  |  |
| Westbound Left Turn | $D(30.6)[<25]\{27\}$ | D (32.8) [<25] \{39\} |
| Westbound Right Turn | A (8.8) [<25] \{63\} | B (11.4) [<25] \{75\} |
| Southbound Left Turn | A (4.3) [<25] \{98\} | B (11.9) [<25] \{<25\} |



## 2023 Existing Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft \} |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 27: Palmetto Bay Rd \& Target Rd (signal) |  |  |
| Overall Intersection | B (16.0) | B (17.6) |
| Eastbound Approach | C (33.3) [<25] \{110\} | D (41.8) [62] \{248\} |
| Westbound Approach | D (37.1) [35] \{151\} | C (29.4) [38] \{149\} |
| Northbound Approach | A (9.7) [28] \{214\} | B (13.3) [71] \{508\} |
| Southbound Approach | B (15.4) [287] \{1309\} | B (13.9) [99] \{742\} |
| 28: Palmetto Bay Rd \& Dunnagans Alley (un-signalized) |  |  |
| Westbound Left Turn | F (184.7) [<25] \{50\} | $F(65.5)[<25]\{75\}$ |
| Westbound Right Turn | A (9.1) [<25] \{84\} | B (13.1) [<25] \{110\} |
| Southbound Left Turn | B (10.4) [112] \{512\} | B (12.5) [<25] \{383\} |
| 29: Palmetto Bay Rd \& Hilton Pkwy (Sea Pines Circle, RAB) |  |  |
| Overall Intersection | E (37.0) | F (51.8) |
| Eastbound Approach | F (208.4) [615] \{814\} | F (51.0) [285] \{770\} |
| Westbound Approach | A (7.5) [<25] \{263\} | F (168.3) [1635] \{1658\} |
| Northbound Approach | A (4.2) [<25] \{223\} | D (30.8) [228] \{738\} |
| Southbound Approach | B (12.9) [194] \{461\} | A (8.4) [85] \{460\} |



## 2045 No Build VISSIM Results

AM Peak Hour Volumes PM Peak Hour Volumes

## 2045 No Build Traffic Volumes

- 0.56\% Annual Growth Rate Applied




AM Peak Hour Volumes PM Peak Hour Volumes

## 2045 No Build Traffic Volumes

- 0.56\% Annual Growth Rate Applied


AM Peak Hour Volumes PM Peak Hour Volumes

## 2045 No Build Traffic Volumes

- 0.56\% Annual Growth Rate Applied




AM Peak Hour Volumes PM Peak Hour Volumes

## 2045 No Build Traffic Volumes



| (16) | $\begin{aligned} & \leftarrow 11 \\ & \leftarrow 1734 \end{aligned}$ |
| :---: | :---: |
| US 278 $1473 \longrightarrow$ |  |

AM Peak Hour Volumes PM Peak Hour Volumes

## 2045 No Build Traffic Volumes

- 0.56\%

Annual
Growth Rate
Applied




AM Peak Hour Volumes PM Peak Hour Volumes

## 2045 No Build Traffic Volumes

- 0.56\% Annual Growth Rate Applied



AM Peak Hour Volumes PM Peak Hour Volumes

## 2045 No Build Traffic Volumes

- 0.56\% Annual Growth Rate Applied



AM Peak Hour Volumes PM Peak Hour Volumes

## 2045 No Build Traffic Volumes

- 0.56\% Annual Growth Rate Applied





## 2045 No Build Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{ Max Queue, ft$\}$ |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 1: Buckingham Plantation Dr \& Bluffton Pkwy (signal) |  |  |
| Overall Intersection | C (22.3) | B (15.9) |
| Eastbound Approach | C (26.6) [123] \{700\} | B (18.5) [43] \{241\} |
| Westbound Approach | B (14.0) [<25] \{168\} | B (14.4) [32] \{218\} |
| Northbound Approach | B (14.1) [<25] \{68\} | B (14.5) [<25] \{53\} |
| Southbound Approach | A (6.6) [<25] \{56\} | A (8.5) [<25] \{83\} |
| 2: Buckingham Plantation Dr/Moss Creek Dr \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | E (74.1) | B (18.6) |
| Eastbound Approach | F (146.0) [1628] \{1658\} | B (16.6) [86] \{567\} |
| Westbound Approach | B (14.1) [37] \{302\} | B (18.4) [86] \{474\} |
| Northbound Approach | C (26.4) [25] \{186\} | D (39.3) [<25] \{91\} |
| Southbound Approach | D (35.8) [<25] \{88\} | C (24.4) [<25] \{119\} |
| 3: Salt Marsh Dr/Moss Creek Village \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | F (54.8) [1075] \{1304\} | $\mathrm{C}(18.8)[<25]\{55\}$ |
| Westbound Left Turn | F (4,646.8) [142] \{214\} | B (12.4) [<25] \{45\} |
| Northbound Approach | B (12.3) [<25] \{63\} | A (7.4) $[<25]\{62\}$ |
| Southbound Approach | D (29.4) [49] \{107\} | A (8.1) $[<25]\{77\}$ |
| 4: Fording Island Rd Ext \& Hilton Pkwy (un-signalized) |  |  |
| Westbound Left Turn | F (3,675.2) [85] \{146\} | C (19.0) [<25] \{34\} |
| Northbound Approach | F (3,448.2) [171] \{189\} | C (18.4) [<25] \{43\} |



Northbound Approach $\quad$ F $(3,448.2)[171]\{189\} \quad$ C (18.4) $[<25]\{43\}$
2045 No Build Analysis includes a 0.56\% annual growth rate as well as signal timing optimizations

## 2045 No Build Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft$\}$ |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 5: Boat Landing Driveway/Wildlife Refuge Driveway \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | C (20.9) [902] \{1658\} | $\mathrm{D}(26.6)[<25]\{<25\}$ |
| Westbound Left Turn | No volumes observed | C (21.4) $[<25]\{<25\}$ |
| Northbound Approach | F (426.9) [<25] \{38\} | C (20.6) [<25] \{47\} |
| Southbound Approach | No volumes observed | $\mathrm{C}(17.5)[<25]\{42\}$ |
| 6: Blue Heron Point Rd \& Hilton Pkwy (un-signalized) |  |  |
| Westbound Left Turn | No volumes observed | F (131.6) [<25] \{<25\} |
| Northbound Approach | F (2,505.2) [214] \{351\} | E (49.9) [<25] \{74\} |
| 7: Crosstree Dr (Windmill Harbour) \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | A (9.5) | B (13.4) |
| Eastbound Approach | B (11.3) [433] \{1093\} | B (10.6) [216] \{1092\} |
| Westbound Approach | A (5.3) [26] \{601\} | A (6.0) [86] \{913\} |
| Northbound Approach | D (41.6) [<25] \{138\} | F (237.4) [193] \{326\} |
| Southbound Approach | A (9.8) $[<25]\{<25\}$ | A (0.0) [<25] \{<25\} |
| 8: Jenkins Rd \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | B (14.4) [<25] \{<25\} | D (32.0) [<25] \{33\} |
| Southbound Approach | D (29.2) [<25] \{47\} | E (43.5) [<25] \{62\} |



## 2045 No Build Results (VISSIM)



## 2045 No Build Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft\} |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 13: Jarvis Park Rd/Wilborn Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (22.3) | C (33.4) |
| Eastbound Approach | B (17.7) [169] \{962\} | B (12.6) [51] \{552\} |
| Westbound Approach | C (21.5) [106] \{673\} | D (42.5) [907] \{1289\} |
| Northbound Approach | E (67.4) [42] \{178\} | F (281.4) [153] \{297\} |
| Southbound Approach | D (36.9) [65] \{225\} | $\mathrm{C}(25.8)$ [32] \{153\} |
| 14: Pembroke Dr/Museum St \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (22.1) | E (55.1) |
| Eastbound Approach | B (17.8) [227] \{1081\} | B (18.9) [99] \{679\} |
| Westbound Approach | B (19.5) [72] \{553\} | D (54.5) [998] \{1279\} |
| Northbound Approach | D (52.8) [71] \{260\} | F (260.2) [397] \{719\} |
| Southbound Approach | C (31.1) [<25] \{87\} | D (43.2) [<25] \{114\} |
| 15: Central Ave \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | B (13.4) $[<25]\{<25\}$ | C (21.6) [87] \{146\} |
| Westbound Left Turn | C (22.4) [<25] \{25\} | C (21.8) [ $<25]\{<25\}$ |
| Northbound Right Turn | $\mathrm{C}(16.2)[<25]\{29\}$ | B (12.6) [<25] \{31\} |
| Southbound Right Turn | B (12.3) [<25] \{44\} | F ( $2,448.4$ [87] \{171\} |
| 16: Hatton Pl/Merchant St \& Hilton Pkwy (un-signalized) |  |  |
| Northbound Right Turn | A (8.1) $[<25]\{<25\}$ | A (6.9) [<25] \{64\} |
| Southbound Right Turn | B (11.0) [<25] \{36\} | F (1,211.0) [123] \{244\} |
| 17: Indigo Run Dr/Whooping Crane Way \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (24.5) | E (66.7) |
| Eastbound Approach | B (19.7) [107] \{764\} | C (34.5) [115] \{636\} |
| Westbound Approach | B (17.3) [53] \{371\} | $\mathrm{E}(75.8)[1341]\{1658\}$ |
| Northbound Approach | E (55.2) [32] \{110\} | E (76.4) [65] \{265\} |
| Southbound Approach | D (47.7) [57] \{203\} | $F(126.4)[327]\{1023\}$ |



## 2045 No Build Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft$\}$ |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 18: Cross Island Pkwy SB Ramp/Gumtree Rd \& Honey Horn Rd (un-signalized) |  |  |
| Eastbound Approach | A (9.7) [<25] \{45\} | B (12.1) [<25] \{72\} |
| 19: Cross Island Pkwy SB Ramp \& Marshland Rd (un-signalized) |  |  |
| Westbound Left Turn | A (4.5) [ $<25$ [ $\{62\}$ | A (2.4) [<25] \{67\} |
| Southbound Left Turn | A (9.8) [<25] \{53\} | B (11.1) [<25] \{79\} |
| Southbound Right Turn | A (6.1) [<25] \{86\} | A (7.9) [<25] \{112\} |
| 20: Cross Island Pkwy NB Ramp \& Marshland Rd (un-signalized) |  |  |
| Eastbound Left Turn | A (1.6) [ 252 ] \{49\} | A (1.5) [<25] $\{<25\}$ |
| Northbound Left Turn | B (11.9) [<25] \{77\} | D (27.8) [46] \{252\} |
| Northbound Right Turn | A (6.0) [<25] \{102\} | A (8.0) [62] \{277\} |



## 2045 No Build Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft $\}$ |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 21: Palmetto Bay Rd \& Bay Pines Rd (un-signalized) |  |  |
| Eastbound Left Turn | No volumes observed | E (48.5) [<25] \{35\} |
| Eastbound Right Turn | C (15.6) [<25] \{<25\} | B (13.9) [<25] \{32\} |
| Northbound Left Turn | B (12.8) [<25] \{<25\} | A (7.8) $[<25]\{31\}$ |
| 22: Palmetto Bay Rd \& Point Comfort Rd/Arrow Rd (signal) |  |  |
| Overall Intersection | B (14.0) | B (18.2) |
| Eastbound Approach | C (30.2) [51] \{187\} | C (29.0) [38] \{164\} |
| Westbound Approach | B (17.3) [<25] \{108\} | C (28.2) [79] \{337\} |
| Northbound Approach | B (12.4) [38] \{311\} | B (15.5) [94] \{596\} |
| Southbound Approach | B (11.6) [91] \{737\} | B (16.3) [90] \{601\} |
| 23: Palmetto Bay Rd \& Genesta St (un-signalized) |  |  |
| Westbound Approach | E (41.7) [<25] \{57\} | D (25.4) [<25] \{53\} |
| Southbound Left Turn | A (5.5) [37] \{255\} | A (8.8) $[<25]\{<25\}$ |
| 24: Palmetto Bay Rd \& Palmetto Business Park Rd (un-signalized) |  |  |
| Westbound Approach | F (65.1) [<25] \{75\} | C (21.0) [<25] \{59\} |
| Southbound Left Turn | A (5.7) [35] \{176\} | B (12.8) [<25] \{<25\} |
| 25: Palmetto Bay Rd \& Bow Cir (un-signalized) |  |  |
| Westbound Left Turn | F (69.4) [46] \{98\} | E (40.7) [<25] \{48\} |
| Westbound Right Turn | A (9.1) [59] \{131\} | B (13.1) [<25] \{81\} |
| Southbound Left Turn | C (16.2) [551] \{1040\} | B (14.1) [<25] \{67\} |
| 26: Palmetto Bay Rd \& Archer Rd (un-signalized) |  |  |
| Westbound Left Turn | F (1,234.0) [40] \{69\} | D (31.8) $[<25]\{34\}$ |
| Westbound Right Turn | B (11.6) [62] \{105\} | B (11.5) [<25] \{70\} |
| Southbound Left Turn | C (16.2) [228] \{469\} | B (12.0) [<25] \{28\} |



## 2045 No Build Results (VISSIM)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ \{Max Queue, ft$\}$ |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 27: Palmetto Bay Rd \& Target Rd (signal) |  |  |
| Overall Intersection | C (26.9) | B (18.6) |
| Eastbound Approach | C (26.5) [<25] \{105\} | D (37.6) [62] \{250\} |
| Westbound Approach | C (34.2) [32] \{157\} | C (26.1) [38] \{144\} |
| Northbound Approach | B (12.5) [39] \{235\} | B (15.4) [83] \{522\} |
| Southbound Approach | C (33.5) [1209] \{1511\} | B (15.6) [129] \{820\} |
| 28: Palmetto Bay Rd \& Dunnagans Alley (un-signalized) |  |  |
| Westbound Left Turn | F (872.0) [62] \{148\} | F (87.7) [<25] \{97\} |
| Westbound Right Turn | E (36.3) [88] \{182\} | B (13.0) [30] \{132\} |
| Southbound Left Turn | C (20.8) [258] \{513\} | B (13.1) [<25] \{416\} |
| 29: Palmetto Bay Rd \& Hilton Pkwy (Sea Pines Circle, RAB) |  |  |
| Overall Intersection | F (60.5) | F (81.8) |
| Eastbound Approach | F (314.7) [808] \{838\} | F (168.2) [805] \{838\} |
| Westbound Approach | B (14.3) [56] \{434\} | F (200.4) [1645] \{1658\} |
| Northbound Approach | A (4.2) [<25] \{207\} | F (61.1) [534] \{794\} |
| Southbound Approach | C (34.4) [297] \{461\} | A (8.2) [88] \{460\} |



## 2045 No Build Synchro Results

## 2045 No Build Results (Synchro)



2045 No Build Analysis includes a $0.56 \%$ annual growth rate as well as signal timing optimizations

## 2045 No Build Results (Synchro)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 5: Boat Landing Driveway/Wildlife Refuge Driveway \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | C (17.8) [<25] <0.01> | F (67.3) [<25] <0.08> |
| Westbound Left Turn | No volumes observed | C (28.7) [<25] <0.01> |
| Northbound Approach | F (50.7) [<25] <0.03> | D (32.3) [<25] <0.09> |
| Southbound Approach | No volumes observed | F (74.9) [<25] <0.26> |
| 6: Blue Heron Point Rd \& Hilton Pkwy (un-signalized) |  |  |
| Westbound Left Turn | No volumes observed | D (30.2) [<25] <0.01> |
| Northbound Approach | F (\$1763.6) [73] <2.35> | F (\$5024.3) [145] <8.33> |
| 7: Crosstree Dr (Windmill Harbour) \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | E (67.8) | F (92.1) |
| Eastbound Approach | F (105.5) [\#2262] <1.19> | C (33.3) [\#1547] <1.00> |
| Westbound Approach | A (5.9) [320] <0.64> | F (133.8) [~2456] <1.26> |
| Northbound Approach | B (14.6) [34] <0.39> | E (77.5) [\#106] <0.86> |
| Southbound Approach | 0 (0.0) [<25]<0.01> | 0 (0.0) [<25] <0.00> |
| 8: Jenkins Rd \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | C (18.3) [<25] <0.03> | F (91.5) [<25] <0.22> |
| Southbound Approach | F (\$1398.4) [65] <1.86> | F (151.3) [73] <.69> |



## 2045 No Build Results (Synchro)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 9: Chamberlin Dr/Squire Pope Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (23.4) | F (144.6) |
| Eastbound Approach | B (15.2) [918] <0.81> | B (12.6) [370] <0.91> |
| Westbound Approach | C (31.1) [625] <0.74> | F (233.2) [m\#2394] <1.49> |
| Northbound Approach | B (13.0) [<25] <0.02> | E (70.6) [67] <0.29> |
| Southbound Approach | E (69.0) [\#286] <0.92> | F (231.6) [\#577] <1.46> |
| 10: Old Wild Horse Rd \& Hilton Pkwy (un-signalized) |  |  |
| Southbound Approach | D (26.7) [40] <0.36> | F(77.9) [<25] <0.20> |
| 11: Spanish Wells Rd/Wild Horse Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | D (37.5) | D (45.1) |
| Eastbound Approach | D (38.7) [\#1280] <1.02> | C (20.5) [381] <0.89> |
| Westbound Approach | C (24.0) [345] <0.91> | D (45.2) [m909] <0.99> |
| Northbound Approach | D (51.5) [214] <0.60> | F (171.7) [\#606] <1.35> |
| Southbound Approach | F (94.1) [\#306] <0.97> | D (49.3) [168] <0.34> |
| 12: Gumtree Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | F (84.2) | F (80.1) |
| Eastbound Approach | F (94.6) [~1146] <1.16> | D (54.0) [\#842] <0.99> |
| Westbound Approach | D (43.1) [\#391]<0.90> | D (44.6) [m539] <0.97> |
| Northbound Approach | F (113.6) [\#372] <1.13> | F (137.9) [\#466] <1.29> |
| Southbound Approach | F (119.0) [\#473] <1.09> | F (178.8) [\#506] <1.33> |



## 2045 No Build Results (Synchro)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 13: Jarvis Park Rd/Wilborn Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (20.2) | E (57.5) |
| Eastbound Approach | B (16.0) [257] <0.90> | A (7.6) [m262] <0.76> |
| Westbound Approach | B (12.0) [306] <0.69> | D (38.2) [\#1335] <1.01> |
| Northbound Approach | F (82.7) [136] <0.58> | F (259.8) [\#208] <1.33> |
| Southbound Approach | D (52.5) [249] <0.73> | F (250.2) [\#358] <1.61> |
| 14: Pembroke Dr/Museum St \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (27.8) | D (38.3) |
| Eastbound Approach | B (15.6) [\#1252] <0.91> | B (13.7) [351] <0.71> |
| Westbound Approach | D (36.7) [681] <0.55> | D (47.7) [1072] <0.89> |
| Northbound Approach | E (73.6) [254] <0.76> | F (86.1) [\#355] <0.89> |
| Southbound Approach | C (32.7) [74] <0.36> | D (49.5) [93] <0.68> |
| 15: Central Ave \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | B (11.4) [<25] <0.01> | B (17.2) [<25] <0.02> |
| Westbound Left Turn | C (18.7) [<25] <0.02> | B (13.6) [<25] <0.01> |
| Northbound Right Turn | $\mathrm{C}(21.0)[<25]<0.03>$ | C (15.9) [<25] <0.04> |
| Southbound Right Turn | B (13.8) [<25] <0.06> | C (19.5) [<25] <0.03> |
| 16: Hatton Pl/Merchant St \& Hilton Pkwy (un-signalized) |  |  |
| Northbound Right Turn | C (17.6) [<25] <0.04> | C (16.1) [<25] <0.17> |
| Southbound Right Turn | B (12.7) [<25] <0.02> | C (17.5) [<25] <0.04> |
| 17: Indigo Run Dr/Whooping Crane Way \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | B (18.0) | D (36.4) |
| Eastbound Approach | A (3.5) [<25]<0.79> | B (16.9) [\#297] <0.93> |
| Westbound Approach | B (19.2) [407] <0.51> | D (38.4) [\#951] <0.92> |
| Northbound Approach | E (62.2) [112] <0.52> | E (70.5) [\#245] <0.82> |
| Southbound Approach | D (54.9) [178] <0.71> | E (67.7) [212] <0.86> |



## 2045 No Build Results (Synchro)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft ] < $/ \mathrm{cc}$ ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 18: Cross Island Pkwy SB Ramp/Gumtree Rd \& Honey Horn Rd (un-signalized) |  |  |
| Eastbound Approach | B (11.5) [<25] <0.00> | B (13.8) [<25] <0.19> |
| 19: Cross Island Pkwy SB Ramp \& Marshland Rd (un-signalized) |  |  |
| Westbound Left Turn | A (9.1) [<25] <0.14> | A (8.6) [<25] <0.17> |
| Southbound Left Turn | C (19.7) [<25] <0.1> | E (49.9) [58] <0.49> |
| Southbound Right Turn | A (9.4) [<25] <0.05> | B (14.1) [<25] <0.19> |
| 20: Cross Island Pkwy NB Ramp \& Marshland Rd (un-signalized) |  |  |
| Eastbound Left Turn | A (8.3) [<25] <0.12> | A (8.6) [<25] <0.06> |
| Northbound Left Turn | C (16.2) [<25] <0.21> | D (34.1) [160] <0.75> |
| Northbound Right Turn | A (9.4) [<25] <0.08> | B (10.6) [<25] <0.23> |



## 2045 No Build Results (Synchro)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 21: Palmetto Bay Rd \& Bay Pines Rd (un-signalized) |  |  |
| Eastbound Left Turn | No volumes observed | F (\$454) [50] <0.74> |
| Eastbound Right Turn | C (21.7) [<25] <0.03> | C (16.5) [<25] <0.04> |
| Northbound Left Turn | C (19.4) [<25] <0.03> | B (14.5) [<25] <0.06> |
| 22: Palmetto Bay Rd \& Point Comfort Rd/Arrow Rd (signal) |  |  |
| Overall Intersection | B (17.1) | C (26.9) |
| Eastbound Approach | D (46.0) [171] <0.71> | C (24.5) [102] <0.35> |
| Westbound Approach | C (23.8) [73] <0.44> | E (55.3) [\#376] <0.94> |
| Northbound Approach | A (9.7) [136] <0.37> | C (21.7) [443] <0.86> |
| Southbound Approach | B (15.2) [670] <0.73> | C (24.3) [434] <0.88> |
| 23: Palmetto Bay Rd \& Genesta St (un-signalized) |  |  |
| Westbound Approach | E (46.3) [<25] <0.13> | F (159.5) [30] <0.37> |
| Southbound Left Turn | B (10.0) [<25] <0.02> | C (16.8) [<25] <0.02> |
| 24: Palmetto Bay Rd \& Palmetto Business Park Rd (un-signalized) |  |  |
| Westbound Approach | E (47.3) [<25] <0.13> | F (114.3) [33] <0.37> |
| Southbound Left Turn | B (10.1) [<25] <0.04> | $\mathrm{C}(16.7)[<25]<0.02>$ |
| 25: Palmetto Bay Rd \& Bow Cir (un-signalized) |  |  |
| Westbound Left Turn | D (27.7) [<25] <0.04> | F (54.8) [<25] <0.14> |
| Westbound Right Turn | B (11.6) [<25] <0.01> | C (19.0) [<25] <0.04> |
| Southbound Left Turn | B (10.3) [<25] <0.07> | $\mathrm{C}(16.7)$ [<25] <0.04> |
| 26: Palmetto Bay Rd \& Archer Rd (un-signalized) |  |  |
| Westbound Left Turn | D (29.0) [<25] <0.07> | F (54.8) [<25] <0.14> |
| Westbound Right Turn | B (11.7) [<25] <0.01> | C (19.0) [<25] <0.04> |
| Southbound Left Turn | B (10.5) [<25] <0.08> | C (16.7) [<25] <0.04> |



## 2045 No Build Results (Synchro)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 27: Palmetto Bay Rd \& Target Rd (signal) |  |  |
| Overall Intersection | B (10.8) | B (18.7) |
| Eastbound Approach | C (33.2) [81] <0.31> | D (51.4) [241] <0.82> |
| Westbound Approach | D (48.8) [152] <0.69> | C (30.4) [115] <0.50> |
| Northbound Approach | B (10.9) [240] <0.37> | B (19.0) [585] <0.72> |
| Southbound Approach | A (4.6) [149] <0.69> | A (7.3) [233] <0.66> |
| 28: Palmetto Bay Rd \& Dunnagans Alley (un-signalized) |  |  |
| Westbound Left Turn | D (33.5) [<25] <0.09> | F (55.4) [<25] <0.33> |
| Westbound Right Turn | B (12.0) [<25] <0.02> | $\mathrm{C}(20.0)[<25]<0.24>$ |
| Southbound Left Turn | B (11.5) [<25] <0.21> | B (14.6) [<25] <0.03> |
| 29: Palmetto Bay Rd \& Hilton Pkwy (Sea Pines Circle, RAB) |  |  |
| Overall Intersection | F (56.0) | F (60.3) |
| Eastbound Approach | F (83.3) [375] <1.04> | F (117.9) [550] 0.98> |
| Westbound Approach | $\mathrm{D}(26.1)$ [200] <0.79> | F (92.4) [425] <1.08> |
| Northbound Approach | C (16.4) [125] <0.65> | F (78.2) [525] <0.98> |
| Southbound Approach | F (134.4) [900] <1.23> | F (94.3) [675] <1.13> |



## Preliminary 2045 Alternative 1 Results

2045 Alternative 1. Modified Recommended Traffic Volumes

AM Peak Hour Volumes PM Peak Hour Volumes



## Preliminary 2045 Alternative 1 Results (SimTraffic)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 1: Buckingham Plantation Dr \& Bluffton Pkwy (signal) |  |  |
| Overall Intersection | B (16.9) | B (12.9) |
| Eastbound Approach | B (16.2) [361] <0.54> | A (8.7) [254] <0.50> |
| Westbound Approach | C (20.4) [180] <0.38> | B (12.1) [291] <0.86> |
| Northbound Approach | B (17.1) [49] <0.08> | D (47.7) [77] <0.11> |
| Southbound Approach | A (7.2) [55] <0.18> | C (27.1) [137] <0.23> |
| 2: Buckingham Plantation Dr/Moss Creek Dr \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | B (14.8) | C (19.1) |
| Eastbound Approach | B (13.1) [283] <0.64> | B (14.0) [274] <0.75> |
| Westbound Approach | B (13.9) [223] <0.51> | C (21.1) [348] <0.89> |
| Northbound Approach | C (24.5) [120] <0.56> | D (38.2) [97] <0.74> |
| Southbound Approach | C (24.4) [80] <0.45> | D (27.4) [169] <0.88> |
| 3: Salt Marsh Dr/Moss Creek Village \& Hilton Pkwy (un-signalized) |  |  |
| Eastbound Left Turn | C (18.9) [27] <0.04> | F (65.8) [33] <0.33> |
| Westbound Left Turn | E (44.8) [<25] <0.08> | D (34.9) [<25] <0.22> |
| Northbound Approach | B (14.2) [59] <0.31> | C (11.4) [59] <0.30> |
| Southbound Approach | B (17.0) [91] <0.63> | F (173.4) [305] <2.05> |
| 4: Fording Island Rd Ext \& Hilton Pkwy (un-signalized) |  |  |
| Westbound Left Turn | D (27.4) [<25] <0.06> | B (17.7) [34] <0.13> |
| Northbound Approach | E (37.7) [53] <0.58> | F (99.8) [127] <1.26> |



Northbound Approach
E (37.7) [53] <0.58> F (99.8) [127] <1.26>

## Preliminary 2045 Alternative 1 Results (SimTraffic)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 5: Boat Landing Driveway/Wildlife Refuge Driveway \& Hilton Pkwy (un-signalized) |  |  |
| Northbound Right-Turn | A (3.0) [<25] <0.00> | A (1.3) [<25]<0.00> |
| Southbound Right-Turn | No volumes observed | A (2.9) [<25]<0.00> |
| 6: Blue Heron Point Rd \& Hilton Pkwy (un-signalized) |  |  |
|  | Intersection combined with Crosstree Dr |  |
| 7: Crosstree Dr (Windmill Harbour) \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | B (15.4) | B (14.6) |
| Eastbound Approach | B (18.6) [338] <0.82> | B (11.6) [283] <0.70> |
| Westbound Approach | A (8.4) [166] <0.45> | B (19.6) [280] <0.92> |
| Northbound Approach | D (52.9) [93] <0.35> | E (60.4) [108] <0.72> |
| Southbound Approach | C (31.0) [<25] <0.06> | D (48.9) [30] <0.14> |
| 8: Jenkins Rd \& Hilton Pkwy (un-signalized) |  |  |
|  | Intersection combined with Crosstree Dr |  |
| A: The Crazy Crab \& Hilton Pkwy (un-signalized) |  |  |
| Westbound Left-Turn | F (169.2) [<25] <0.05> | F (77.1) [<25] <0.02> |
| Northbound Right-Turn | F (95.8) [<25] <0.02> | E (39.3) [<25] <0.01> |



## Preliminary 2045 Alternative 1 Results (SimTraffic)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 9: Chamberlin Dr/Squire Pope Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | B (18.1) | C (23.3) |
| Eastbound Approach | B (13.3) [852] <0.75> | B (11.1) [467] <0.71> |
| Westbound Approach | A (3.2) [91] <0.48> | B (18.0) [522] <0.97> |
| Northbound Approach | E (70.6) [263] <0.07> | F (84.1) [\#464] <0.29> |
| Southbound Approach | D (52.9) [151] <0.58> | $\mathrm{E}(65.6)$ [281] <0.70> |
| 10: Old Wild Horse Rd \& Hilton Pkwy (un-signalized) |  |  |
| Southbound Approach | C (17.0) [74] <0.36> | E (43.2) [32] <0.20> |
| 11: Spanish Wells Rd/Wild Horse Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (26.0) | C (20.9) |
| Eastbound Approach | B (11.7) [354] <0.99> | B (10.1) [216] <0.85> |
| Westbound Approach | A (8.7) [350] <0.91> | C (28.3) [1107] <0.99> |
| Northbound Approach | E (73.5) [459] <0.66> | E (70.9) [\#609] <0.95> |
| Southbound Approach | E (58.6) [411] <0.76> | E (57.7) [430] <0.77> |
| 12: Gumtree Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | D (38.8) | D (38.4) |
| Eastbound Approach | C (32.3) [655] <0.77> | C (31.3) [489] <0.93> |
| Westbound Approach | D (40.4) [310] <0.75> | D (43.6) [554] <0.97> |
| Northbound Approach | E (57.4) [375] <0.41> | D (54.5) [319] <0.93> |
| Southbound Approach | E (60.8) [293] <0.87> | E (74.3) [298] <0.90> |



## Preliminary 2045 Alternative 2 Results

AM Peak Hour Volumes PM Peak Hour Volumes
2045 Alternative 2. Bowties at Spanish Wells and Squire Pope Traffic Volumes




2045 Alternative 2. Bowties at Spanish Wells and Squire Pope Traffic Volumes

AM Peak Hour Volumes PM Peak Hour Volumes



## Preliminary 2045 Alternative 2 Results (SimTraffic)

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 9: Chamberlin Dr/Squire Pope Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | B (16.3) | B (13.6) |
| Eastbound Approach | B (15.8) [394] <0.82> | B (14.7) [301] <0.65> |
| Westbound Approach | B (11.5) [253] <0.45> | B (11.3) [507] <0.95> |
| Northbound Approach | E (60.4) [277] <0.78> | D (44.0) [535] <0.94> |
| Southbound Approach | B (12.2) [87] <0.62> | D (51.8) [369] <0.82> |
| 9A: Squire Pope North Bowtie (Roundabout) |  |  |
| Overall Intersection | A (4.5) | A (5.5) |
| Eastbound Approach | A (3.7) [<25] | A (4.4) [<25] |
| Westbound Approach | A (3.4) [<25] | A (3.9) [<25] |
| Northbound Approach | A (4.2) [25] | A (5.1) [25] |
| Southbound Approach | A (4.7) [25] | A (5.9) [50] |
| 9B: Chamberlin Dr South Bowtie (Roundabout) |  |  |
| Overall Intersection | A (4.2) | A (4.9) |
| Eastbound Approach | A (3.4) [<25] | A (3.8) [<25] |
| Northbound Approach | A (3.5) [<25] | A (4.0) [<25] |
| Southbound Approach | A (4.2) [25] | A (5.0) [25] |
| 10: Old Wild Horse Rd \& Hilton Pkwy (un-signalized) |  |  |
| Southbound Approach | C (17.0) [80] <0.36> | F (51.5) [33] <0.20> |



## Preliminary 2045 Alternative 2 Results (SimTraffic)



## Preliminary 2045 Alternative 3 Results

AM Peak Hour Volumes PM Peak Hour Volumes
2045 Alternative 3. Echelon at Spanish Wells Traffic Volumes


## Preliminary 2045 Alternative 3 Results (SimTraffic) Echelon at Spanish Wells

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft ] <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 9: Chamberlin Dr/Squire Pope Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | B (11.2) | C (20.4) |
| Eastbound Approach | B (12.4) [236] <0.75> | C (24.4) [478] <0.89> |
| Westbound Approach | A (7.7) [159] <0.48> | B (15.5) [332] <0.94> |
| Northbound Approach | D (41.3) [29] <0.03> | E (62.3) [77] <0.28> |
| Southbound Approach | B (16.7) [102] <0.57> | C (26.9) [274] <0.83> |
| 10: Old Wild Horse Rd \& Hilton Pkwy (un-signalized) |  |  |
| Southbound Approach | A (4.2) [37] <0.36> | A (4.8) [<25] <0.17> |
| 11A: Wild Horse Rd \& Hilton Pkwy (signal) - Echelon |  |  |
| Overall Intersection | B (10.8) | B (12.8) |
| Westbound Approach | A (7.2) [134] <0.38> | B (11.5) [249] <0.82> |
| Southbound Approach | C (34.5) [138] <0.58> | C (32.8) [131] <0.53> |
| 11B: Spanish Wells Rd \& Hilton Pkwy (signal) - Echelon |  |  |
| Overall Intersection | B (18.3) | B (12.4) |
| Eastbound Approach | B (17.3) [474] <0.95> | A (9.4) [288] <0.75> |
| Northbound Approach | C (28.1) [147] <0.72> | C (29.1) [233] <0.77> |
| 12: Gumtree Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | D (44.6) | D (44.2) |
| Eastbound Approach | D (53.1) [449] <0.88> | D (42.5) [406] <0.93> |
| Westbound Approach | C (30.0) [479] <0.38> | D (42.1) [824] <0.97> |
| Northbound Approach | C (33.3) [213] <0.75> | D (46.0) [356] <0.93> |
| Southbound Approach | E (59.5) [239] <0.86> | D (54.4) [274] <0.90> |



AM Peak Hour Volumes PM Peak Hour Volumes
2045 Alternative 3. Echelon at Squire Pope Traffic Volumes


## Preliminary 2045 Alternative 3 Results (SimTraffic) Echelon at Squire Pope

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft$]$ <v/c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 9A: Squire Pope Rd \& Hilton Pkwy (signal) - Echelon |  |  |
| Overall Intersection | A (3.6) | C (35.0) |
| Westbound Approach | A (2.2) [57] <0.51> | A (8.4) [258]<0.95> |
| Southbound Approach | B (6.6) [156] <0.62> | $\mathrm{F}(250.5)[858]<1.02>$ |
| 9B: Chamberlin Dr \& Hilton Pkwy (signal) - Echelon |  |  |
| Overall Intersection | C (25.9) | A (4.6) |
| Eastbound Approach | C (25.9) [263] <0.66> | A (4.1) [167] <0.54> |
| Northbound Approach | C (26.9) [151] <0.05> | D (37.9) [66] <0.19> |
| 10: Old Wild Horse Rd \& Hilton Pkwy (un-signalized) |  |  |
| Southbound Approach | C (17.6) [85] <0.36> | B (12.7) [<25] <0.17> |
| 11: Spanish Wells Rd/Wild Horse Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | D (35.5) | D (39.6) |
| Eastbound Approach | D (45.0) [906] <0.99> | B (17.4) [369] <0.67> |
| Westbound Approach | B (11.8) [168] <0.69> | D (52.5) [984] <0.93> |
| Northbound Approach | D (43.2) [167] <0.51> | E (67.8) [239] <0.90> |
| Southbound Approach | E (61.3) [214] <0.98> | D (47.1) [166] <0.37> |
| 12: Gumtree Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | D (35.5) | D (42.4) |
| Eastbound Approach | D (44.9) [464] <0.87> | D (35.6) [399] <0.87> |
| Westbound Approach | B (17.1) [204] <0.41> | D (42.8) [801] <0.83> |
| Northbound Approach | C (28.7) [202] <0.75> | D (43.3) [364] <0.89> |
| Southbound Approach | D (52.7) [230] <0.77> | E (55.1) [260] <0.77> |



## 2045 Alternative 3. Center Turn Overpass at Spanish Wells Traffic Volumes



AM Peak Hour Volumes PM Peak Hour Volumes


| (12) | $\begin{aligned} & \leftarrow 369 \\ & \leftarrow 1745 \\ & \leftarrow 220 \end{aligned}$ |
| :---: | :---: |
| $\begin{array}{r} \text { US } 278 \\ 141 \\ 1204 \\ 26 \end{array}$ |  |

## Preliminary 2045 Alternative 3 Results (SimTraffic) Center Turn Overpass at Spanish Wells



2045 Alternative 3. Center Turn Overpass at Squire Pope Traffic Volumes


AM Peak Hour Volumes PM Peak Hour Volumes



## Preliminary 2045 Alternative 3 Results (SimTraffic) Center Turn Overpass at Squire Pope

| Intersection \& Movements | LOS (Delay, sec) [95 ${ }^{\text {th }}$ Queue Length, ft ] < $/$ /c ratio> |  |
| :---: | :---: | :---: |
|  | AM Peak Hour | PM Peak Hour |
| 9A: Chamberlin Dr/Squire Pope Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | A (3.2) | A (9.3) |
| Eastbound Approach | A (3.5) [40] <0.78> | A (4.8) [139] <0.64> |
| Westbound Approach | A (2.0) [42] <0.43> | A (7.4) [234] <0.93> |
| Northbound Approach | D (37.3) [21] <0.05> | E (68.1) [67] <0.11> |
| Southbound Approach | A (6.1) [149] <0.77> | D (48.3) [387] <0.89> |
| 9B: Chamberlin Dr/Squire Pope Rd \& Hilton Pkwy (signal) - Center Turn Overpass |  |  |
| Overall Intersection | B (17.2) | B (10.0) |
| Eastbound Approach | B (15.9) [108] <0.21> | A (3.9) [70] <0.65> |
| Westbound Approach | B (10.6) [<25] <0.00> | A (4.1) [<25] <0.95> |
| Northbound Approach | B (18.8) [<25] <0.00> | E (56.2) [<25] <0.94> |
| Southbound Approach | C (21.8) [64] <0.10> | D (40.9) [77] <0.82> |
| 10: Old Wild Horse Rd \& Hilton Pkwy (un-signalized) |  |  |
| Southbound Approach | C (17.4) [77] <0.36> | E (42.4) [29] <0.20> |
| 11: Spanish Wells Rd/Wild Horse Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | C (32.4) | C (31.3) |
| Eastbound Approach | D (39.0) [771] <0.98> | B (17.6) [369] <0.68> |
| Westbound Approach | B (12.6) [149] <0.91> | C (22.1) [446] <0.99> |
| Northbound Approach | D (44.0) [162] <0.64> | F (157.4) [766] <1.28> |
| Southbound Approach | E (62.8) [220] <0.82> | E (57.9) [152] <0.85> |
| 12: Gumtree Rd \& Hilton Pkwy (signal) |  |  |
| Overall Intersection | D (35.8) | D (36.4) |
| Eastbound Approach | D (45.7) [473] <0.87> | C (32.5) [387] <0.93> |
| Westbound Approach | B (17.4) [263] <0.41> | C (33.5) [651] <0.97> |
| Northbound Approach | C (28.3) [199] <0.75> | D (36.4) [277] <0.93> |
| Southbound Approach | $\mathrm{D}(52.7)[228]<0.77>$ | $E(56.5)[262]<0.90>$ |



